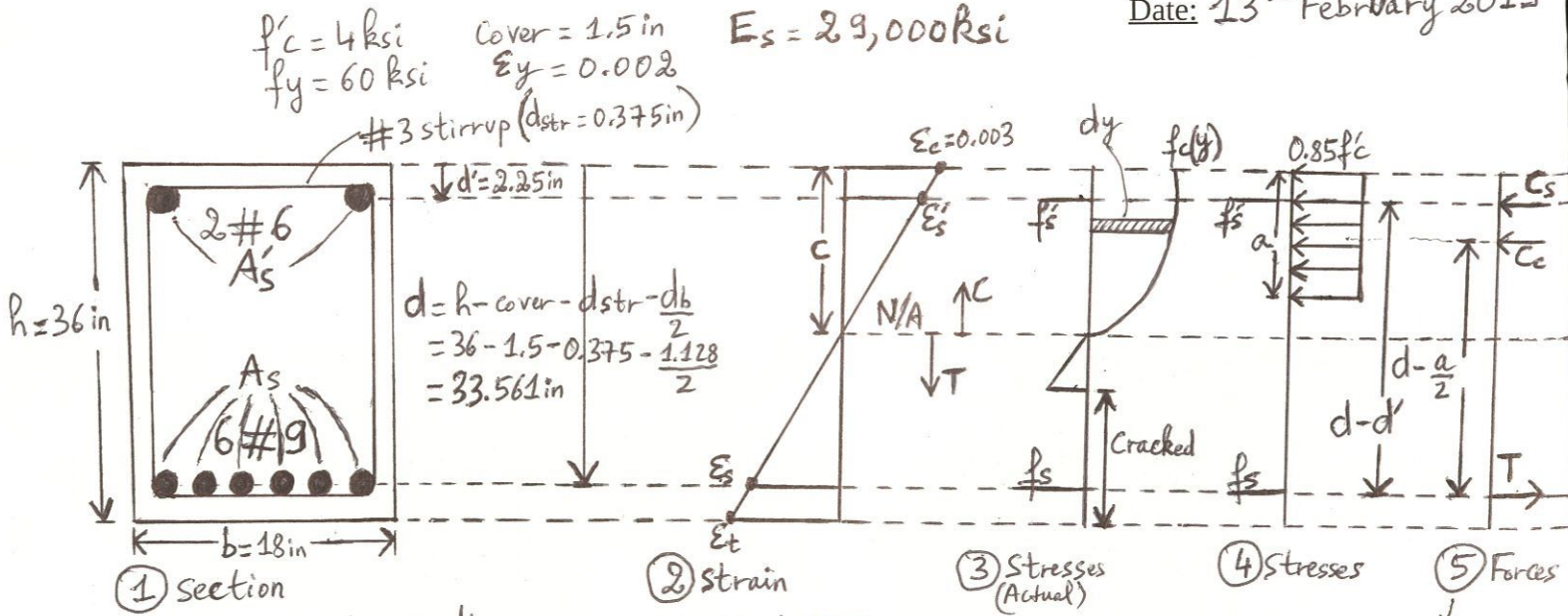


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$$d' = \text{cover} + d_{str} + \frac{d_b}{2} = 1.5 \text{ in} + 0.375 \text{ in} + \frac{0.75 \text{ in}}{2} = 2.25 \text{ in}$$

$$\#9 \rightarrow A_b = 1 \text{ in}^2 ; A_s = (1 \text{ in}^2)(6) = 6 \text{ in}^2$$

$$\#6 \rightarrow A_b = 0.44 \text{ in}^2 ; A_s' = (0.44 \text{ in}^2)(2) = 0.88 \text{ in}^2$$

$$T = A_s f_y = (6 \text{ in}^2)(60 \text{ ksi}) = 360 \text{ kips}$$

$$C_c = 0.85 f'_c a b = 0.85 (4 \text{ ksi})(a)(18 \text{ in}) = (61.2a) \text{ kips} ; a = \beta_1 c = 0.85c \Rightarrow c = \frac{a}{0.85}$$

Assuming both tension and compression steel yields

$$\epsilon_s' \gg \epsilon_y \text{ \& } \epsilon_s \gg \epsilon_y ; f_s' = f_s \text{ \& } f_s = f_y$$

$$C_s' = A_s' f_s' = (0.88 \text{ in}^2)(60 \text{ ksi}) = 52.8 \text{ kips}$$

from (5); $T = C_c + C_s \Rightarrow 360 \text{ kips} = (61.2a) \text{ kips} + (52.8 \text{ kips}) \Rightarrow c = \frac{a}{0.85} = \frac{5.0196 \text{ in}}{0.85} = 5.905 \text{ in}$

Check strain

$$\epsilon_s' = \epsilon_c \left(\frac{c - d'}{c} \right) = (0.003) \left[\frac{5.905 - 2.25}{5.905} \right] = 1.86 \times 10^{-3} = 0.00186 < \epsilon_y$$

$$\epsilon_s = \frac{\epsilon_c (d - c)}{c} = (0.003) \left[\frac{33.561 - 5.905}{5.905} \right] = 0.0141 > \epsilon_y \Rightarrow \text{Assumption } \epsilon_s > \epsilon_y \text{ is correct}$$

Assumption $\epsilon_s' \gg \epsilon_y$ not valid



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Compression steel did not yield because $\epsilon'_s < \epsilon_{y2}$, therefore

$$f'_s = (E_s)(\epsilon'_s) \neq f_y$$

$$\epsilon'_s = \epsilon_c \left(\frac{c-d'}{c} \right) = 0.003 \left[\frac{\left(\frac{a}{0.85} \right) - 2.25 \text{ in}}{\left(\frac{a}{0.85} \right)} \right] = \frac{0.003a - 0.0057375}{a}$$

$$f'_s = (29,000 \text{ ksi}) \left[\frac{0.003a - 0.0057375}{a} \right] = \left(\frac{87a - 166.3875}{a} \right) \text{ kips}$$

$$T = 360 \text{ kips}$$

$$C_c = (61.2a) \text{ kips} ; C_s = A'_s f'_s = (0.88 \text{ in}^2) \left[\frac{87a - 166.3875}{a} \right] = \frac{76.56a - 146.421}{a}$$

$$T = C_c + C_s \Rightarrow 360 \text{ kips} = (61.2a) \text{ kips} + \left(\frac{76.56a - 146.421}{a} \right) \text{ kips}$$

$$a = 5.10 \text{ in} \Rightarrow c = \frac{a}{0.85} = \frac{5.10}{0.85} = 6 \text{ in}$$

check strains again

$$\epsilon'_s = \epsilon_c \left(\frac{c-d'}{c} \right) = 0.003 \left[\frac{6 - 2.25}{6} \right] = 1.875 \times 10^{-3} = 0.001875 < \epsilon_{y2}$$

$$\epsilon_s = \epsilon_c \left(\frac{d-c}{c} \right) = 0.003 \left[\frac{33.561 - 6 \text{ in}}{6 \text{ in}} \right] = 0.0138 > \epsilon_y \rightarrow \text{Assumption } \epsilon_s > \epsilon_y \text{ is still valid}$$

Assumption $\epsilon'_s < \epsilon_{y2}$ is valid

compute ultimate moment capacity

$$+\left(\sum M_N @ T \right) = 0 \text{ from } \textcircled{5}$$

$$M_N = C_c \left(d - \frac{a}{2} \right) + C_s (d - d') = 61.2(5.10 \text{ in}) \left[33.561 - \frac{5.10 \text{ in}}{2} \right] +$$

$$\frac{76.56(5.10) - 146.421}{5.10} \left[33.561 - 2.25 \right]$$

$$M_N = 11,177.38 \text{ kips-in} \left(\frac{1 \text{ ft}}{12 \text{ in}} \right) = 931.45 \text{ kips-ft}$$

$$\phi M_N = (0.9)(931.45) = 838.3 \text{ kips-ft}$$